

### BALLARD TRUSS

Client: Walk in - Mayer
Job Name: Home of my own
Job #: 105780
Location: , Prescott AZ

By signing below, I agree that I have reviewed this layout and the attached truss drawings and found them to be in conformance to my needs for this project, even if it they have deviated from the plans.

Disclaimer: This Truss Placement Diagram was not created by an engineer, but rather by the Ballard Truss Staff and is purely to be used as an installation guide and does not require a seal. Complete truss engineering and analysis can be found on the Truss Design Drawings which may be sealed by the Truss Designer.



MiTek USA, Inc. MiTek USA, Inc. 400 Sunrise Avenue, Suite 270 Roseville, CA 95661 Telephone 916-755-3571

Re: 105780

Yavapai County 1 Bedroom

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Ballard-Mesa, Snow.

Pages or sheets covered by this seal: R64673571 thru R64673589

My license renewal date for the state of Arizona is March 31, 2022.

Arizona COA: 11906-0

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

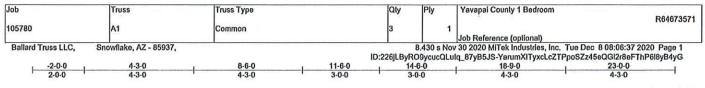


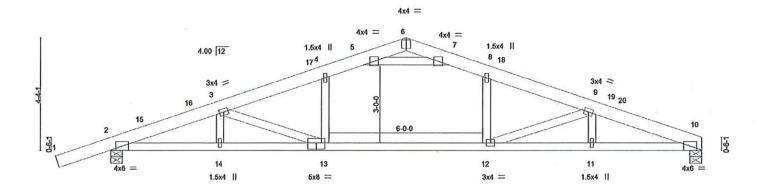
REVIEWED FOR DESIGN CRITERIA ONLY

December 8,2020

Dyer, Cecil

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





	4-3-0 4-3-0	8-6-0 4-3-0		14-6-0 6-0-0		<del> </del>	18-9-0 4-3-0		-0-0 3-0
Plate Offsets (X,Y)	[2:0-2-6,Edge], [10:0-2-6,E	dge], [13:0-3-12,0-3-0	0]						
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/I	2-0-0 1.15 1.15 YES IPI2014	CSI. TC 0.54 BC 0.69 WB 0.67 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.34 11-12 -0.44 11-12 0.10 10	l/defl >804 >612 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 100 lb	GRIP 185/144 FT = 10%

LUMBER-

TOP CHORD 2x6 SPF 1650F 1.5E

2x4 SPF 1650F 1.5E **BOT CHORD** 

2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 4-4-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 10=0-5-8, 2=0-5-8

Max Horz 2=95(LC 18) Max Uplift 10=-97(LC 11), 2=-209(LC 10) Max Grav 10=1502(LC 22), 2=1749(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3542/291, 3-4=-2697/221, 4-5=-2484/235, 7-8=-2478/242, 8-9=-2718/237,

9-10=-3684/362

referenced standard ANSI/TPI 1.

2-14=-223/3236, 13-14=-223/3236, 12-13=-101/2486, 11-12=-290/3403, 10-11=-290/3403 8-12=-5/431, 9-12=-1127/254, 4-13=0/387, 3-13=-973/210, 5-7=-2558/273 **BOT CHORD** 

WEBS

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp CyEncipsed VIEWED FOR MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 11-6-0, Exterior(2R) (11-6-0, Exterior(2R) MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
2) TCLL: ASCE 7-16; P(=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

ONLY

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

5) 150.0lb AC unit load placed on the top chord, 11-6-0 from left end, supported at two points, 3-0-0 apart.

6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

  \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 10 and 209 lb uplift at 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

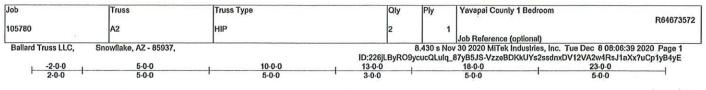


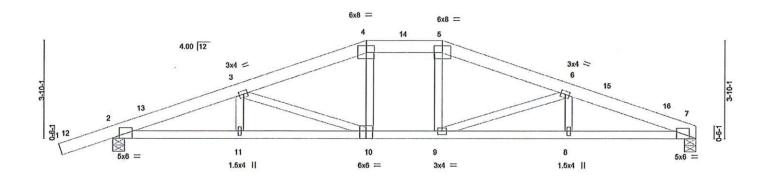
December 8,2020

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MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Dasign valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to provent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of furuses and furuse systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







<b>⊢</b>	5-0-0 5-0-0	1	10-0-0 5-0-0		13-0-0 3-0-0	-		18-0-0 5-0-0		23-0-0	
Plate Offsets (X,Y)	[2:0-3-2,Edge], [7:0-3-2,Edge	], [10:0-3-0,E									
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI	2-0-0 1,15 1,15 YES 2014	CSI. TC BC WB Matrix	0.28 0.67 0.71 -SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	In -0.23 -0.31 0.12	(loc) 8-9 8-9 7	l/defl >999 >861 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 100 lb	GRIP 185/144 FT = 10%

LUMBER-

TOP CHORD 2x6 SPF 1650F 1.5E

BOT CHORD 2x4 SPF 1650F 1.5E

WEBS 2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud BRACING-TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 4-2-7 oc purlins.

Rigid ceiling directly applied or 9-10-12 oc bracing.

ONLY

REACTIONS.

(size) 7=0-5-8, 2=0-5-8 Max Horz 2=86(LC 14) Max Uplift 7=-183(LC 11), 2=-295(LC 10)

Max Grav 7=1738(LC 35), 2=2119(LC 35)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3846/595, 3-4=-2852/500, 4-5=-2613/508, 5-6=-2857/509, 6-7=-3984/610

BOT CHORD 2-11=-510/3469, 10-11=-510/3469, 9-10=-346/2613, 8-9=-528/3647, 7-8=-528/3647

WEBS 3-10=-1010/186, 4-10=-4/405, 5-9=-22/445, 6-9=-1155/221

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed: MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 10-0-0, Exterior(2E) 10-0-0, Exterior(2E) 13-0-0 to 18-0-0, Interior(1) 18-0-0 to 22-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33

  2) TCLL: ASCE 7-16; P(=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Cl=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on everhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.

referenced standard ANSI/TPI 1.

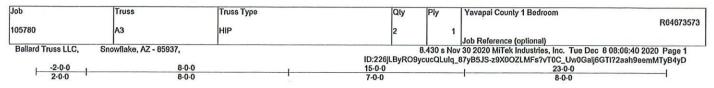
- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is
  the responsibility of the fabricator to increase plate sizes to account for these factors.
   7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 7 and 295 lb uplift at joint 2. 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and



December 8,2020

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 ray. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an Individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss was and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and furse systems, see ANSITPII Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





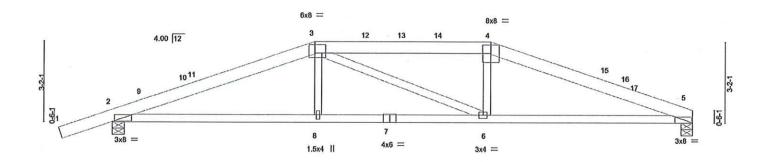


Plate Offsets (X,Y) [	8-0-0 8-0-0 3:0-5-4,0-4-0], [5:0-0-6,Edge]		15-0-0 7-0-0		1		23-0-0 8-0-0	
LOADING (ps) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.76 BC 0.74 WB 0.34 Matrix-SH	DEFL. in Vert(LL) -0.20 Vert(CT) -0.33 Horz(CT) 0.09	5-6 5-6	l/defl >999 >822 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 89 lb	GRIP 185/144 FT = 10%

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

TOP CHORD 2x6 SPF 1650F 1.5E

**BOT CHORD** 2x4 SPF 1650F 1.5E

2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud WEBS

(size) 5=0-5-8, 2=0-5-8

Max Horz 2=73(LC 14)

Max Uplift 5=-195(LC 11), 2=-307(LC 10) Max Grav 5=1527(LC 35), 2=1907(LC 35)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3139/563, 3-4=-2893/578, 4-5=-3177/568

**BOT CHORD** 2-8=-451/2822, 6-8=-447/2830, 5-6=-448/2892 3-8=0/293, 3-6=-320/418, 4-6=0/292 **WEBS** 

### NOTES-

- 1) Wind: ASCE 7-16; Vull=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed:

  MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 8-0-0, Exterior(2R) 8-0-0 to 19-12-1/0; EWED FOR Interior(1) 12-11-6 to 15-0-0, Exterior(2R) 15-0-0 to 19-11-6, Interior(1) 19-11-6 to 22-9-4 zone; cantilever left and right exposed; end FOR vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate and FOR EXECUTED IN CRITERIA

  2) TCLL: ASCE 7-16; P(=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ci=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.
6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 ib uplift at joint 5 and 307 ib uplift at joint 2.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502,11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 3-2-15 oc purlins.

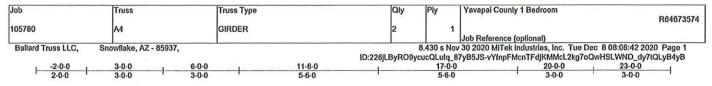
Rigid ceiling directly applied or 10-0-0 oc bracing.

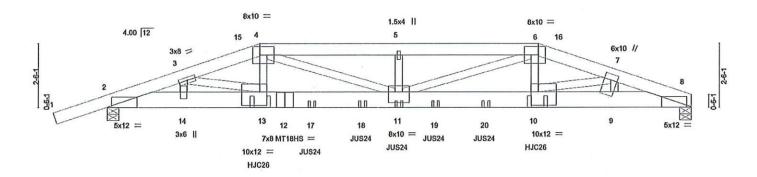
ONLY

December 8,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL 7473 ray, 5/19/2020 REFORE USE Design valid for use only with MITok® connectors. This design is based only upon parameters shown and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal fujury and propert dydange. For general guidence regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see — ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801







	3-0-0	6-0-0 3-0-0	11-6-0 5-6-0			17-0-0 5-6-0			3-0-0	3-0-0
Plate Offsets (X,Y)			12,0-4-0], [7:0-2-12,0-3-0], [8	:0-2-0,Edge], [	10:0-3-8,0		11:0-5-0	,0-5-0], [13:0		0.0.0
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING Plate Grip Lumber De Rep Stress Code IRC	DOL 1.15 OL 1.15	CSI. TC 0.87 BC 0.68 WB 0.78 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.50 -0.66 0.11	(loc) 11 11 8	l/defl >544 >410 n/a	L/d 240 180 n/a	PLATES MT20 MT18HS Weight: 144 lb	GRIP 185/144 244/190 FT = 10%

BRACING-TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x6 SPF 2100F 1.8E

**BOT CHORD** 2x8 SP 2400F 2.0E

WEBS 2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud \*Except\*

4-11,6-11: 2x4 SPF 1650F 1.5E

REACTIONS.

(size) 8=0-5-8, 2=0-5-8 Max Horz 2=69(LC 46)

Max Uplift 8=-496(LC 7), 2=-608(LC 6)

Max Grav 8=3447(LC 18), 2=3694(LC 17)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-8256/1098, 3-4=-9800/1378, 4-5=-11883/1700, 5-6=-11883/1700, 6-7=-9887/1426,

7-8=-8456/1210

**BOT CHORD** 2-14=-1029/7708, 13-14=-1029/7708, 11-13=-1284/9418, 10-11=-1305/9502,

9-10=-1112/7912, 8-9=-1112/7912

**WEBS** 3-14=-883/192, 3-13=-254/1815, 4-13=-161/1604, 4-11=-405/2617, 5-11=-734/216,

6-11=-385/2586, 6-10=-182/1669, 7-10=-249/1757, 7-9=-825/162

### NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C Enclosed; MWFRS (envelope) gable end zone: cantilever left and right exceed a part of the control of the co POL=1.33 plate ONLY

2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.0; Cs=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is

the responsibility of the fabricator to increase plate sizes to account for these factors.

7) All plates are MT20 plates unless otherwise indicated.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 8 and 608 lb uplift at joint 2.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

12) Use USP HJC26 (With 16-16d nails into Girder & 10d nails into Truss) or equivalent spaced at 10-11-4 oc max. starting at 6-0-6 from the left end to 16-11-10 to connect truss(es) to front face of bottom chord.

13) Use USP JUS24 (With 4-10d nails into Girder & 2-10d nails into Truss) or equivalent spaced at 2-0-0 oc max. starting at 8-0-12

Continued by lesigned to 14-11-4 to connect truss(es) to front face of bottom chord.

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ANSITPH Quality Criterie, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 1-11-10 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REVIEWED FOR

December 8,2020



Job	Truss	Truss Type	Qly	Ply	Yavapai County 1 Bedroom	Delegaeza
105780	A4	GIRDER	2	1		R64673574
	- CON				Job Reference (optional)	
D-1117 110	D			100 11		0.00.00 10.0000 0 0

Ballard Truss LLC,

Snowflake, AZ - 85937,

8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:42 2020 Page 2 ID:226jLByRO9ycucQLulq\_87yB5JS-vY/npFMcnTFdjKMMcL2kg7oQwHSLWND\_dy7tQLyB4yB

### NOTES-

14) Fill all nail holes where hanger is in contact with lumber.

15) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-100, 4-6=-100, 6-8=-100, 2-8=-20

Concentrated Loads (lb)

Vert: 13=-945(F) 11=-407(F) 10=-945(F) 17=-407(F) 18=-407(F) 19=-407(F) 20=-407(F)

REVIEWED FOR **DESIGN CRITERIA** ONLY

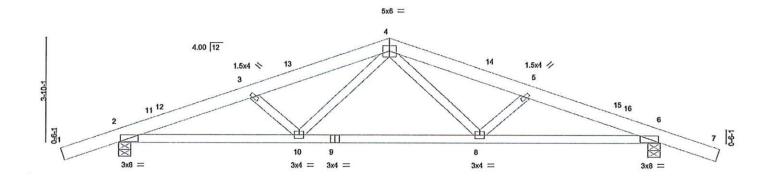
WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	Yavapai County 1 Bedroom	
105780	B1	соммон	1	1		R64673575
					Job Reference (optional)	
Ballard Truss LLC,	Snowflake, AZ - 85937,			3.430 s Nov	30 2020 MiTek Industries, Inc. Tue Dec 8 0	8:06:43 2020 Page 1
			ID:226jLByRO9y	cucQLulq_	87yB5JS-NkC91bNEYnNUKUxZA2ZzCLLkvh	rzFxV7rcsQyoyB4yA
-2-0-0	5-0-0	10-0-0	, 1	5-0-0	20-0-0	22-0-0
2-0-0	5-0-0	5-0-0	,	5-0-0	5-0-0	2-0-0

Scale = 1:38.9



<b>—</b>	6-8-0 6-8-0		-4-0 8-0	1	20-0-0 6-8-0	
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES           Code IRC2018/TPI2014	CSI. TC 0.28 BC 0.47 WB 0.33 Matrix-SH	DEFL. in (lo Vert(LL) -0.11 8- Vert(CT) -0.19 8- Horz(CT) 0.06	·10 >999 240	MT20	GRIP 185/144 FT = 10%

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x6 SPF 1650F 1.5E BOT CHORD 2x4 SPF 1650F 1.5E

WEBS 2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

REACTIONS. (size) 2=0-5-8, 6=0-5-8 Max Horz 2=75(LC 18)

Max Uplift 2=-260(LC 10), 6=-260(LC 11)

Max Grav 2=1521(LC 21), 6=1521(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2795/439, 3-4=-2294/377, 4-5=-2294/376, 5-6=-2795/438

BOT CHORD 2-10=-324/2525, 8-10=-181/1613, 6-8=-344/2525

WEBS 4-8=-52/766, 5-8=-626/176, 4-10=-51/766, 3-10=-626/175

### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 10-0-0, Exterior(2R) 10-0-0 to 13-6-0 Interior(1) 13-6-0 to 22-0-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members are though the property of the second gust) FOR & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=100; EG I GN CRITERIA
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.
- 5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 260 lb uplift at joint 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 5-4-1 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 8,2020

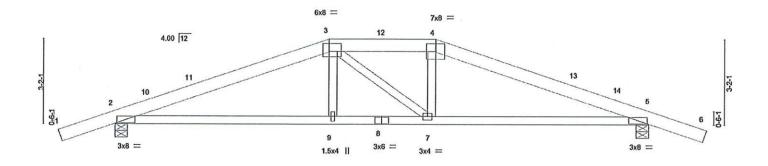
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. &19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing Indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and fruss systems, see ANSIITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job		Truss	Truss Type	Qly	Ply	Yavapai County 1 Bedroom	
105780		B2	HIP	1	1		R64673576
						Job Reference (optional)	
Ballard Truss L	LC, Sn	owflake, AZ - 85937,		8	.430 s Nov	30 2020 MiTek Industries, Inc. Tue Dec 8 08	:06:44 2020 Page 1
				ID:226JLByRO9yc	ucQLulq_8	7yB5JS-rwmXEwOsJ5VLyeWljm4ClYuqK59K	_RhH4GczVEyB4y9
-2-0		8-0-0	i	12-0-0		20-0-0	22-0-0
2-0-	) '	8-0-0		4-0-0		8-0-0	2-0-0

Scale = 1:39.6



<u> </u>	8-0-0 8-0-0	1	12-0-0 4-0-0	1		20-0-0 8-0-0	
Plate Offsets (X,Y) [3:	0-5-4,0-3-8]						
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.62 BC 0.59 WB 0.14 Matrix-SH		in (loc) -0.14 2-9 -0.26 2-9 0.07 5	>999 2 >912 1	L/d PLATES 40 MT20 80 n/a Weight: 81 lb	GRIP 185/144 FT = 10%

BRACING-

TOP CHORD

BOT CHORD

LUMBER.

REACTIONS.

TOP CHORD 2x6 SPF 1650F 1.5F

**BOT CHORD** 2x4 SPF 1650F 1.5E

WEBS

2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

(size) 2=0-5-8, 5=0-5-8

Max Horz 2=63(LC 14)

Max Uplift 2=-274(LC 10), 5=-274(LC 11)

Max Grav 2=1793(LC 35), 5=1793(LC 35)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2524/476, 3-4=-2194/522, 4-5=-2527/499

**BOT CHORD** 2-9=-334/2184, 7-9=-331/2191, 5-7=-367/2187

WEBS 3-7=-314/321, 4-7=-47/293

### NOTES-

- NOTES1) Wind: ASCE 7-16; Vull=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed:

  MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 8-0-0, Exterior(2E) 8-0-0 to 16-11-6, Interior(1) 16-11-6 to 22-0-14 zone; cantilever left and right exposed; end vertical left and hight exposed:

  Exterior(2R) 12-0-0 to 16-11-6, Interior(1) 16-11-6 to 22-0-14 zone; cantilever left and right exposed; end vertical left and hight exposed:

  CRITERIOR

  CRITERI exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33

  TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Cl=1.10

  Libbelanced specification of the control of the con ONLY
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on dverhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.

referenced standard ANSI/TPI 1.

- 6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 274 lb uplift at joint 5. 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and



Structural wood sheathing directly applied or 4-3-8 oc purlins.

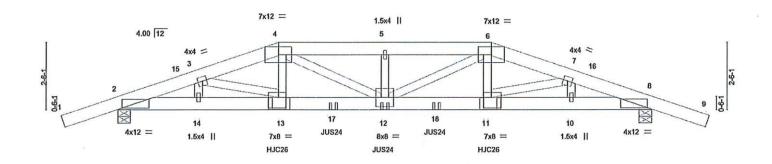
Rigid ceiling directly applied or 10-0-0 oc bracing.

A WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 (ey. 5/19/2020 BEFORE USE, Design valid for use only with MITGNO connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to provent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and furse systems, see — ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type		Qly	Ply	Yavapai County 1 Bedi	room	word for the Grance (1997)
105780	ВЗ	Hip Girder		1	1			R64673577
						Job Reference (optional	1)	
Ballard Truss LLC,	Snowflake, AZ - 8593	7,				30 2020 MiTek Industrie		
				ID:226jLByR	09ycucQLi	ulq_87yB5JS-oJuHfcP7c	qll3Bxf8rB7gqzzCduov	vSEeZYa54Z7yB4y7
-2-0-0	3-0-0	6-0-0	10-0-0	14-0-0		17-0-0	20-0-0	22-0-0
2-0-0	3-0-0	3-0-0	4-0-0	4-0-0		3-0-0	3-0-0	2-0-0

Scale = 1:39.6



H	3-0-0	6-0-0 3-0-0	10-0-0 4-0-0	14-0-0	17-0-0 3-0-0	3-0-0	
Plate Offsets (X,Y)	[11:0-3-8,0-4-12], [13:0-3-	8,0-4-12]					
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/	2-0-0 1.15 1.15 NO TPI2014	CSI. TC 0.51 BC 0.71 WB 0.62 Matrix-SH		nc) I/defl L/d 12 >717 240 12 >536 180 8 n/a n/a		BRIP 85/144 FT = 10%

LUMBER-

TOP CHORD 2x6 SPF 1650F 1.5E

**BOT CHORD** 2x6 SPF 2100F 1.8E

2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud WERS

BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 2-9-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REVIEWED FOR

REACTIONS. (size) 2=0-5-8, 8=0-5-8

Max Horz 2=-51(LC 7)

Max Uplift 2=-511(LC 6), 8=-511(LC 7) Max Grav 2=3137(LC 31), 8=3137(LC 31)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-6773/849, 3-4=-7491/1020, 4-5=-8241/1157, 5-6=-8241/1157, 6-7=-7491/1021,

7-8=-6773/852

**BOT CHORD** 2-14=-769/6249, 13-14=-769/6249, 12-13=-921/7183, 11-12=-888/7183, 10-11=-737/6249,

8-10=-737/6249

3-14=-385/117, 3-13=-179/1089, 4-13=-119/1227, 4-12=-221/1451, 5-12=-502/157,

6-12=-221/1451, 6-11=-120/1227, 7-11=-181/1089, 7-10=-385/116

### NOTES-

WEBS

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C Fixes IGN CRITERIA MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber OL=1.33 plate
- 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.

7) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.

- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 511 lb uplift at joint 2 and 511 lb uplift at joint 8.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Use USP HJC26 (With 16-16d nails into Girder & 10d nails into Truss) or equivalent spaced at 7-11-4 oc max. starting at 6-0-6 from the left end to 13-11-10 to connect truss(es) to front face of bottom chord.
- 12) Use USP JUS24 (With 4-10d nails into Girder & 2-10d nails into Truss) or equivalent spaced at 1-11-4 oc max. starting at 8-0-12 from the left end to 11-11-4 to connect truss(es) to front face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

Ochlinules & PAN SASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITE&D connectors. This design is based only upon parameters show, and is for an individual building component, not a russ system. Before use, the building designer must verify the opplicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to provent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, reaction and bracing of trusses and furse systems, see 
ANSITPH Quality Criterie, DSB-89 and BCSI Building Component Safety Information evaluable from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qly	Ply	Yavapai County 1 Bedroom	5
105780	В3	Hip Girder	1	1		R64673577
D. C.			Y.		Job Reference (optional)	
Ballard Truss LLC,	Snowflake, AZ - 85937,		8	.430 s Nov	30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:46 2020	Page 2

8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:46 2020 Page 2 ID:226jLByRO9ycucQLulq\_87yB5JS-oJuHfcP7qll3Bxf8rB7gqzzCduowSEeZYa54Z7yB4y7

LOAD CASE(S) Standard

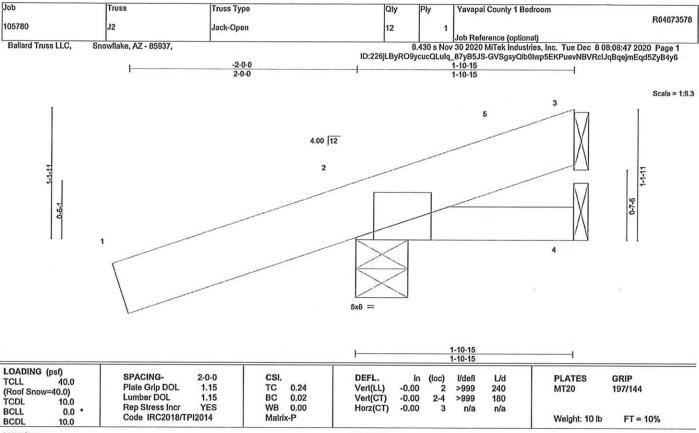
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
Vert: 1-4=-100, 4-6=-100, 6-9=-100, 2-8=-20
Concentrated Loads (lb)
Vert: 13=-945(F) 12=-407(F) 11=-945(F) 17=-407(F) 18=-407(F)

REVIEWED FOR DESIGN CRITERIA ONLY

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LUMBER-

TOP CHORD 2x6 SPF 1650F 1.5E BOT CHORD 2x4 SPF 1650F 1.5E BRACING-

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 1-10-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

ONLY

REACTIONS.

(size) 3=Mechanical, 2=0-5-8, 4=Mechanical

Max Horz 2=66(LC 10)

Max Uplift 3=-199(LC 20), 2=-168(LC 10)

Max Grav 3=25(LC 10), 2=636(LC 20), 4=37(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vull=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 1-10-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Cl=1.10
- 3) Unbalanced snow loads have been considered for this design.
- Onbalanced snow loads have been considered for this design.
   This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on perhaps VIEWED FOR non-concurrent with other live loads.
- 5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection confliction CRITERIA the responsibility of the fabricator to increase plate sizes to account for these factors. 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit botween the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 3 and 168 lb uplift at
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Job Truss Truss Type Yavapai County 1 Bedroom Qly Ply R64673579 105780 J4 Jack-Open 10 1 Job Reference (optional) 8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:47 2020 Page 1 Ballard Truss LLC, Snowflake, AZ - 85937, ID:226jLByRO9ycucQLulq\_87yB5JS-GVSgsyQlb0twp5EKPuevNBVQ?llbBqeJmEqd5ZyB4y6 3-10-15 Scale = 1:12.6 4.00 12 1-9-11 2 4x4 = Plate Offsets (X,Y)--[2:0-3-6,Edge] LOADING (psf) SPACING-2-0-0 CSI DEFL PLATES GRIP TCLL Plate Grip DOL TC BC 1.15 0.28 Vert(LL) -0.01 2-4 >999 240 MT20 197/144 (Roof Snow 40.0) Lumber DOL 1.15 0.10 Vert(CT) -0.022-4 >999 180 TCDL 10.0 Rep Stress Incr WB 0.00 YES Horz(CT) -0.003 n/a n/a BCLL 0.0 Code IRC2018/TPI2014 Matrix-P Weight: 16 lb FT = 10% BCDL 10.0 LUMBER-BRACING-2x6 SPF 1650F 1.5E TOP CHORD TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins. 2x4 SPF 1650F 1.5E **BOT CHORD** BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-5-8, 4=Mechanical

Max Horz 2=94(LC 10)

Max Uplift 3=-96(LC 20), 2=-164(LC 10)

Max Grav 3=154(LC 21), 2=726(LC 21), 4=72(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vull=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 3-10-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Pertially Exp.; Ce=1.0; Cs=1<u>.00: Cl=1 10</u>
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhange VIEWED FOR non-concurrent with other live loads. non-concurrent with other live loads.

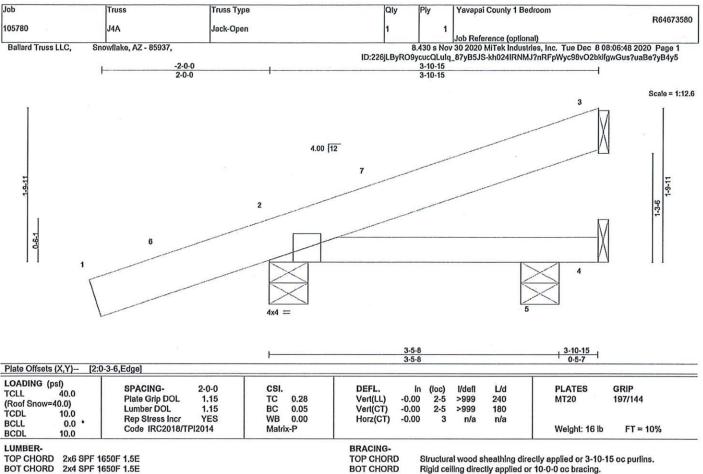
  5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is
- the responsibility of the fabricator to increase plate sizes to account for these factors.
- 6) This truss has been designed for a 10.0 psf bollom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 takey 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3 and 164 lb uplift at
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



December 8,2020

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**BOT CHORD** 2x4 SPF 1650F 1.5E

(size) 3=Mechanical, 2=0-5-8, 4=Mechanical, 5=0-5-8 Max Horz 2=94(LC 10) Max Uplift 3=-96(LC 20), 2=-172(LC 10), 4=-48(LC 5)

Max Grav 3=154(LC 21), 2=714(LC 21), 5=146(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

REACTIONS.

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 3-10-3 zone; cantillever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate
- TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1\_00: Cl=1\_10
- 3) Unbalanced snow loads have been considered for this design.
- 3) Unbalanced snow loads have been considered for this design.
  4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on dverhangs VIEWED FOR
- 5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these fort
- 6) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 takey 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3, 172 lb uplift at joint 2 and 48 lb uplift at joint 4.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



December 8,2020

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Job Truss Truss Type Yavapai County 1 Bedroom R64673581 105780 J4B Jack-Open Job Reference (optional) Ballard Truss LLC. Snowflake, AZ - 85937, 8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:49 2020 Page 1 ID:226jLByRO9ycucQLulq\_87yB5JS-CuaQHeS?7d7e2POiWJgNScboe6\_3fj80EYJkASyB4y4 Scale = 1:11 8 4.00 12 LOADING (psf) SPACING-2-0-0 CSI. DEFL. **Vdefl** GRIP in (loc) L/d **PLATES** 40.0 TCLL Plate Grip DOL 1.15 TC 0.14 Vert(LL) -0.01 >999 240 197/144 1-3 MT20 (Roof Snow 40.0) Lumber DOL 1.15 BC 0.10 Vert(CT) -0.02 1-3 >999 180 TCDL Rep Stress Incr YES WB 0.00 Horz(CT) -0.00 n/a BCLL 0.0 Code IRC2018/TPI2014 Matrix-P Weight: 12 lb FT = 10% BCDL LUMBER-BRACING-TOP CHORD 2x6 SPF 1650F 1.5E TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2x4 SPF 1650F 1.5E **BOT CHORD** REACTIONS. (size) 1=0-5-8, 2=Mechanical, 3=Mechanical Max Horz 1=61(LC 10) Max Uplift 1=-23(LC 10), 2=-68(LC 10) Max Grav 1=282(LC 20), 2=246(LC 20), 3=72(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; canllever left and right exposed; end vertical left and right exposed; crossed; end vertical left and right exposed; crossed; crossed
- 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. **REVIEWED FOR** 

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall page N CRITERIA will fit between the bottom chord and any other members. 7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 1 and 68 lb uplift at loint 2.

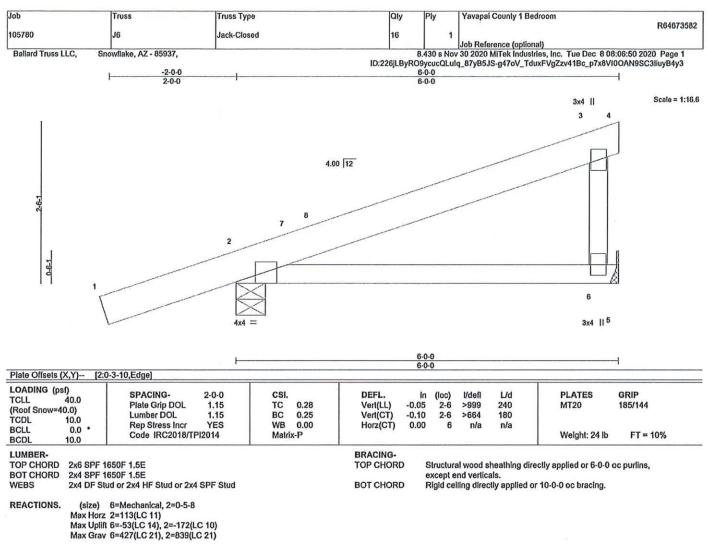
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



ONLY

A WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE, Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss wab and/or chord members only. Additional temporary and permanent bracing is always required for slability and to prevent ocllapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems, see \*\*ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2870 Crain Highway, Suite 203 Waldorf, MD 20601





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-6=-366/245

### NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 6-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOI =1 33 plate 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00, RELYOLEWED FOR 3) Unbalanced snow loads have been considered for this design

DESIGN CRITERIA

3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.

6) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 6 and 172 lb uplift at

10) This truss is designed in accordance with the 2018 international Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

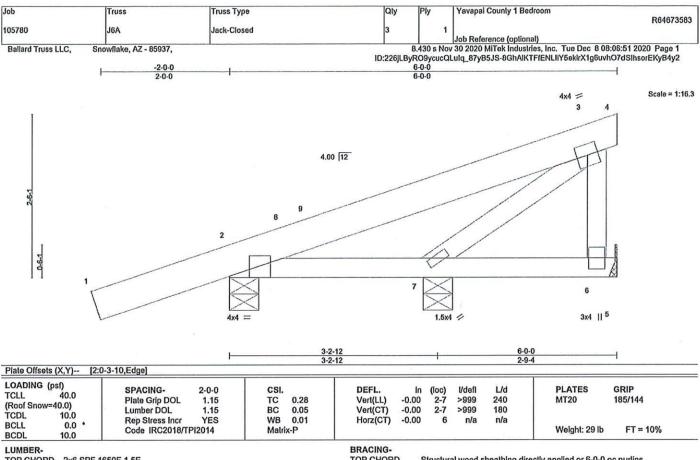


ONLY

December 8,2020

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TOP CHORD 2x6 SPF 1650F 1.5E

2x4 SPF 1650F 1.5E **BOT CHORD** 

2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

ONLY

REACTIONS. (size) 6=Mechanical, 2=0-5-8, 7=0-5-8

Max Horz 2=113(LC 11)
Max Uplift 6=-76(LC 14), 2=-194(LC 10)
Max Grav 6=380(LC 21), 2=794(LC 21), 7=139(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-6=-353/250

### NOTES-

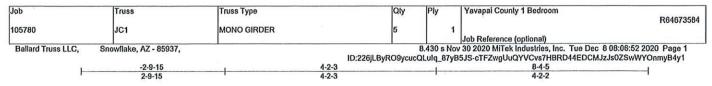
- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-14 to 1-5-2, Interior(1) 1-5-2 to 6-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOI =1 33 plate TCLL: ASCE 7-16; P(=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00, Cs=1.0 DESIGN CRITERIA
- 3) Unbalanced snow loads have been considered for this design.
  4) This truss has been designed for greater of min roof live load of 16.0 psf or 2.00 times flat roof load of 40.0 psf on or non-concurrent with other live loads.
- 5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 6) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 6 and 194 lb uplift at joint 2.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



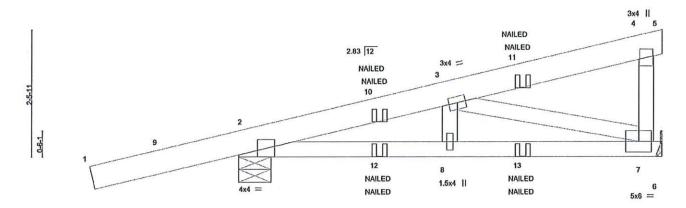
December 8,2020

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Scale = 1:20.8



	H	4-2-3 4-2-3	8-4-5 4-2-2			
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. DEFL. In TC 0.62 Vert(LL) -0.02 BC 0.20 Vert(CT) -0.03 WB 0.28 Horz(CT) 0.01 Matrix-P	(loc)	10%		

LUMBER-

TOP CHORD 2x6 SPF 1650F 1.5E

2x4 SPF 1650F 1.5E **BOT CHORD** 

2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

BRACING-TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

DESIGN CRITERIA ONLY

except end verticals.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(size) 7=Mechanical, 2=0-7-12

Max Horz 2=106(LC 7) Max Uplift 7=-47(LC 10), 2=-202(LC 6)

Max Grav 7=567(LC 17), 2=1014(LC 17)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-862/253, 4-7=-331/73

**BOT CHORD** 2-8=-320/725, 7-8=-320/725

WEBS

3-7=-754/333

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DQI =1 33 plate grip DOL=1.33
2) TCLL: ASCE 7-16; P[=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00, ReEVIEWED FOR

3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on non-concurrent with other live loads.

5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It ls the responsibility of the fabricator to increase plate sizes to account for these factors.

6) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at Joint 7 and 202 lb uplift at
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.

12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-100, 4-5=-100, 2-6=-20



December 8,2020

### Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Yavapai County 1 Bedroom
105780	JC1	MONO GIRDER	5	1	R64673584
		- A - 1975			Job Reference (optional)
Ballard Truss LLC,	Snowflake, AZ - 85937,			3.430 s Nov	30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:52 2020 Page 2
			ID:226jLByRO9ycucO	Lulq_87yB	5JS-cTFZwgUuQYVCvs7HBRD44EDCMJzJs0ZSwWYOnmyB4y1

LOAD CASE(S) Standard

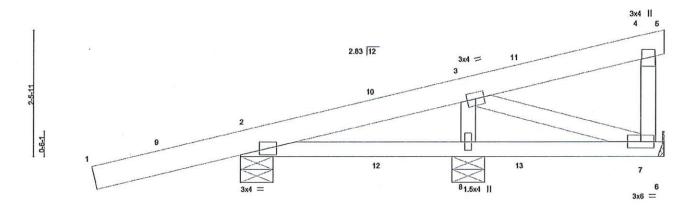
Concentrated Loads (lb)
Vert: 10=68(F=34, B=34) 11=-38(F=-19, B=-19) 13=-16(F=-8, B=-8)

REVIEWED FOR DESIGN CRITERIA ONLY



Yavapai County 1 Bedroom Job Truss Type Truss Qly R64673585 105780 JC2 MONO GIRDER Job Reference (optional) 8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:53 2020 Page 1 Ballard Truss LLC, Snowflake, AZ - 85937, ID:226jLByRO9ycucQLulq\_87yB5JS-4fpx7?VWBsd3X0lUl9lJcSlMMjL0bWCb9AHyJDyB4y0 4-6-1 4-6-1

Scale = 1:20.8



	<del></del>	8-4-5 3-10-4	
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. DEFL. TC 0.60 Verl(LL) BC 0.10 Verl(CT) WB 0.12 Horz(CT) Matrix-P	) -0.02 2-8 >999 180

LUMBER-

2x6 SPF 1650F 1.5E TOP CHORD

**BOT CHORD** 2x4 SPF 1650F 1.5E

WEBS 2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud **BRACING-**TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

(size) 7=Mechanical, 2=0-7-12, 8=0-7-12

Max Horz 2=106(LC 7)

Max Uplift 7=-73(LC 6), 2=-220(LC 6), 8=-227(LC 16) Max Grav 7=334(LC 31), 2=896(LC 16), 8=478(LC 17)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD

4-7=-314/78

WFRS 3-8=-419/287

### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate
- 2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00 REVIEWED FOR 3) Unbalanced snow loads have been considered for this design.

  4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on cvening SIGN CRITERIA
- 5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

non-concurrent with other live loads.

- 8) Refer to girder(s) for truss to truss connections.

  9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 73 lb uplift at joint 7, 220 lb uplift 2, 2 and 227 lb uplift at joint 8.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 82 lb down and 231 lb up at 2-9-8, 82 lb down and 231 lb up at 2-9-8, and 184 lb down and 81 lb up at 5-7-7, and 98 lb down and 52 lb up at 5-7-7 on top chord, and 2 lb down at 2-9-8, 2 lb down at 2-9-8, and 19 lb down at 5-7-7, and 52 lb up at 5-7-7 on boltom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15



December 8,2020

Continued on page 2

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Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an Individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of Individual Tuss was and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible porsonal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and furse systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801



Job	Truss	Truss Type	Qty	Ply	Yavapai County 1 Bedroom R64673585
105780	JC2	MONO GIRDER	1	1	
					Job Reference (optional)
Ballard Truss LLC, Sr	owflake, AZ - 85937,		8	.430 s Nov	30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:53 2020 Page 2

8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:53 2020 Page 2 ID:226jLByRO9ycucQLulq\_87yB5JS-4fpx7?VWBsd3X0IUI9JJcSIMMJL0bWCb9AHyJDyB4y0

LOAD CASE(S) Standard
Uniform Loads (plf)
Vert: 1-4=-100, 4-5=-100, 2-6=-20

Concentrated Loads (Ib)
Vert: 10=68(F=34, B=34) 11=-124(F=-19, B=-105) 13=45(F=52, B=-8)

REVIEWED FOR DESIGN CRITERIA ONLY

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rov. 5/19/2020 BEFORE USE.

Dasign valid for use only with MITek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a russ system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven buckling of Individual russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of Individual russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and furses systems, see

\*\*ANSITIPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Scale: 3/8"=1"

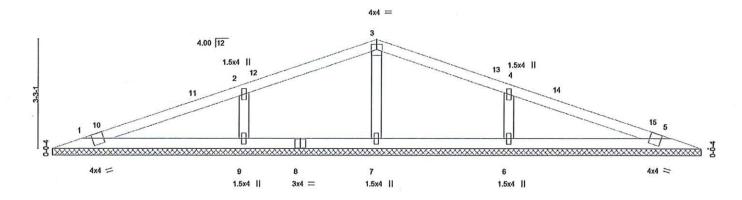


Plate Offsets (X,Y)-- [4:0-0-0,0-0-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defl 1/d PLATES GRIP TCLL 40.0 Plate Grip DOL 1.15 TC 0.44 185/144 Vert(LL) n/a n/a 999 **MT20** (Roof Snow=40.0) 1.15 BC 0.22 Lumber DOL Vert(CT) 999 n/a n/a TCDL 10.0 Rep Stress Incr YES WB 0.17 Horz(CT) 0.00 5 n/a n/a 0.0 BCLL Code IRC2018/TPI2014 Weight: 51 lb FT = 10% BCDL 10.0

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SPF 1650F 1.5E

BOT CHORD 2x4 SPF 1650F 1.5E OTHERS 2x4 DF Slud or 2x4 HF Stud or 2x4 SPF Slud

REACTIONS. All bearings 19-6-4.

(lb) - Max Horz 1=-55(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-134(LC 14), 6=-134(LC 15)

Max Grav All reactions 250 lb or less at joint(s) except 1=292(LC 20), 5=292(LC 21), 7=353(LC 1), 9=879(LC 20),

6=880(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-7=-307/69, 2-9=-730/197, 4-6=-730/197

### NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-11-5 to 4-5-5, Interior(1) 4-5-5 to 9-9-2, Exterior(2R) 9-9-2 to 13-3-2 Interior(1) 13-3-2 to 18-6-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and touch the sum of the

3) Unbalanced snow loads have been considered for this design.

4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is

ONLY
the responsibility of the fabricator to increase plate sizes to account for these factors.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 7) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=134, 6=134.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate bits design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and fruss systems, see 

ANSITIPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20501



Truss	Truss Type	Qly	Ply	Yavapai County 1 Bedroom	
V2	DROP GABLE	1	1		R64673587
		100	-	Job Reference (optional)	
owflake, AZ - 85937,					
		ID:226JLByRO9	cucQLulq_	87yB5JS-12xhYhWmJTtnmKsstannhtrnzW1u	J3QeucUm3N5yB4y_
7-9-2		1		15-6-4	
7-9-2				7-9-2	
1	V2 nowflake, AZ - 85937, 7-9-2	V2 DROP GABLE nowlake, AZ - 85937, 7-9-2	V2 DROP GABLE 1  nowllake, AZ - 85937,  7-9-2	V2 DROP GABLE 1 1 1 nowllake, AZ - 85937, ID:226jLByRO9ycucQLulq_	V2 DROP GABLE 1 1 1 Job Reference (optional)  nowllake, AZ - 85937,    B.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 0   ID:226jLByRO9ycucQLulq_87yB5JS-12xhYhWm]TtnmKsstannhtrnzW1t

Scale = 1:25.3

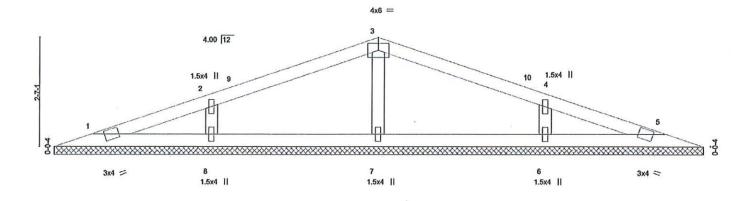


Plate Offsets (X Y) (4:	Plate Offsets (X,Y) [4:0-0-0,0-0-0]										
LOADING (ps)   TCLL	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.28 BC 0.08 WB 0.13 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (le n/a n/a 0.00	loc) I/defl - n/a - n/a 5 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 39 lb	GRIP 185/144 FT = 10%			

15-6-4

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SPF 1650F 1.5E

**BOT CHORD** 2x4 SPF 1650F 1.5E OTHERS 2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

REACTIONS. All bearings 15-6-4.

(lb) - Max Horz 1=43(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 7 except 8=-104(LC 14), 6=-104(LC 15)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=435(LC 1), 8=668(LC 20), 6=668(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-7=-355/117, 2-8=-582/198, 4-6=-583/198

### NOTES-

1) Wind: ASCE 7-16; Vull=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; WHIRS (envelope) gable end zone and C-C Exterior(£E) 0-11-5 to 4-5-5, Interior(1) 4-5-5 to 7-9-2, Exterior(2R) 7-9-2 to 11-3-2, Interior(1) 11-3-2 to 14-6-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and force & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33 1.00 REVIEWED FOR

2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs DESIGN CRITERIA 3) Unbalanced snow loads have been considered for this design.

4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection of the responsibility of the fabricator to increase plate sizes to account for these factors.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf boltom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (jt=lb) 8=104, 6=104.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

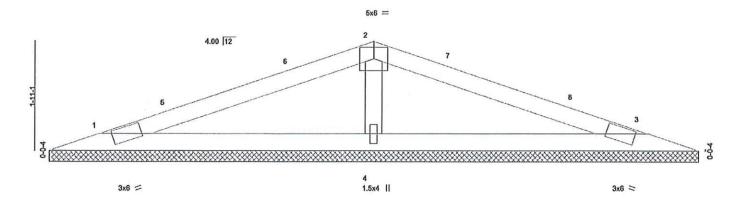
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December 8,2020

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This down trins And included in the received the received in the received in



Job	Truss	Truss Type	Qly	Ply	Yavapai County 1 Bedroom
105780	V3	DROP GABLE	1	1	R64673588
1000000	1-	BEST TANK BEST CALIFORN		271	Job Reference (optional)
Ballard Truss LLC, Sno	owflake, AZ - 85937,				v 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:56 2020 Page 1
		ID:226	LByRO9yc	cQLulq_8	B7yB5JS-VEV3I1XOUn0eOUQ3QHI0E4Nv_wJFotD2r8WcvYyB4xz
1	5-9-2			10 0000	11-6-4
1.00	5-9-2				5-9-2



pl.					11-6-4						
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri	0.45 0.32 0.11 x-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	I/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 26 lb	GRIP 185/144 FT = 10%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SPF 1650F 1.5E

**BOT CHORD** 2x4 SPF 1650F 1.5E

**OTHERS** 2x4 DF Stud or 2x4 HF Stud or 2x4 SPF Stud

REACTIONS. (size) 1=11-6-4, 3=11-6-4, 4=11-6-4

Max Horz 1=30(LC 14) Max Uplift 1=-45(LC 10), 3=-48(LC 15), 4=-59(LC 10) Max Grav 1=351(LC 20), 3=351(LC 21), 4=661(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**WEBS** 

### NOTES-

- 1) Wind: ASCE 7-16; Vull=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-11-5 to 4-5-5, Interior(1) 4-5-5 to 5-9-2, Exterior(2R) 5-9-2 to 9-3-2, Interior(1) 9-3-2 to 10-6-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
  2) TCLL: ASCE 7-16; Pf=40.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00 REVIEWED FOR
- 3) Unbalanced snow loads have been considered for this design.
- 4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection 📶 📆 📆 🕻 🗬 СРЕГЕТВЕТА the responsibility of the fabricator to increase plate sizes to account for these factors.

  5) Gable requires continuous bottom chord bearing.

  6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 takby 2-0-0 wide will fit between the bottom chord and any other members.
- B) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

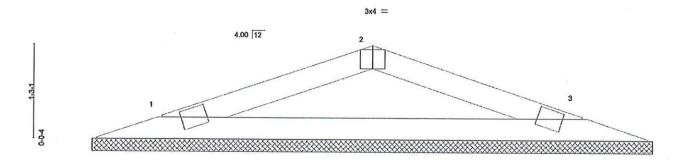
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A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rav. 5/19/2020 BEFORE USE. Design valid for use only with MITak® connectors. This design is based only upon parameter share, which will be component, not a truss system. Before use, the building designer must verify the applicability of design parameters and propely incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to provent collapse with possible porsonal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and furse systems, see ANSITIPII Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qly Ply Yavapai County 1 Bedroom
105780	V4	DROP GABLE	1 1 R64673589
			Job Reference (optional)
Ballard Truss LLC,	Snowflake, AZ - 85937,		8.430 s Nov 30 2020 MiTek Industries, Inc. Tue Dec 8 08:06:56 2020 Page 1
			ID:226jLByRO9ycucQLuIq_87yB5JS-VEV3I1XOUn0eQUQ3QHI0E4NzmwKxous2r8WcyYyB4xz
j.		3.9.2	7-6-4
,		3-9-2	3-9-2

5.



3x4 =

3x4 =

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

ONLY

-			7-6-4 7-6-4						
Plate Offsets (X,Y) [2:	0-2-0,Edge]		7-0-4						· ·
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.21 BC 0.28 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	I/defi n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 15 lb	GRIP 197/144 FT = 10%

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

TOP CHORD 2x4 SPF 1650F 1.5E **BOT CHORD** 

2x4 SPF 1650F 1.5E

(size) 1=7-6-4, 3=7-6-4 Max Horz 1=18(LC 18)

Max Uplift 1=-44(LC 10), 3=-44(LC 11) Max Grav 1=366(LC 20), 3=366(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-476/356, 2-3=-476/361

**BOT CHORD** 1-3=-296/417

### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right

- exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33

  2) TCLL: ASCE 7-16; P[=40.0 pst (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00 CELVIEWED FOR

  3) Unbalanced snow loads have been considered for this design.

  4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection confidences increase plate sizes to account for these factors.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 takey 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



December 8,2020

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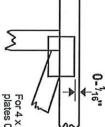


## Symbols

# PLATE LOCATION AND ORIENTATION



and fully embed teeth. offsets are indicated. Center plate on joint unless x, y Apply plates to both sides of truss Dimensions are in ft-in-sixteenths.



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

œ

o

connector plates. required direction of slots in This symbol indicates the

Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE



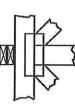
width measured perpendicular to slots. Second dimension is the length parallel to slots. The first dimension is the plate

# LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the if indicated output. Use T or I bracing

### BEARING



Min size shown is for crushing only. reaction section indicates joint number where bearings occur. Indicates location where bearings (supports) occur. Icons vary but

## Industry Standards:

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.

DSB-89: ANSI/TPI1:

Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses. Building Component Safety Information,

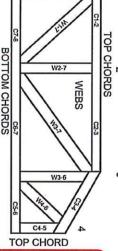
# Numbering System

648

dimensions shown in ft-in-sixteenths

(Drawings not to scale)





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1 established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses. A wide trust spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing slouid be considered.
- RI stack mat designer, erection supervisor, property owner and a stripther interested parties.
- Cut members to bear tightly against each other.
- © Place plales on each face of truss at each Lipint and winbed fully. Knots and wane at joint Cocations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- use with fire retardant, preservative treated, or green lumber. Unless expressly noted, this design is not applicable for
- 10. Camber is a non-structural consideration and is the camber for dead load deflection. of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions ndicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.